

A Stochastic Formulation to Assess the Environmental Impact of the Life-Cycle of Engineering Systems

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Abstract

This paper proposes a Stochastic Life-Cycle Assessment (LCA) as a general stochastic formulation to quantify the environmental impact of an engineering system. The assessment is performed in terms of embodied energy, energy consumption, and carbon footprint during the service life of the system. At the same time, its ability to resist a hazard is considered. The proposed formulation is general and takes into account the effects of shocks (robustness) and gradual deterioration (durability), assessing the GHG emissions due to repair or reconstruction after an undesired event. The formulation is used in an example of four-story reinforced concrete (RC) office building, whose construction site is relevantly seismic. Two different seismic design levels are compared.

1 Introduction

Over the last few years, governments around the world have been trying to reduce emissions for limiting global warming. Many governments aim to go beyond the Kyoto targets Protocol (Ochsendorf 2012). The building sector accounts for 30-40 % of all the primary energy consumption and is responsible for 40-50 % of GHG emissions across the world (Ramesh et al.

29 2010; Luo et al. 2016). As a result, in recent years there has been a great growth of green
30 building rating systems such as the well-known LEED protocol (Leadership in Energy and
31 Environmental Design by U.S. Green Building Council).

32 The Intergovernmental Panel on Climate Change (IPCC) suggests reducing emissions from
33 buildings by limiting energy consumption, easing renewable energy, and monitoring non-CO₂
34 emissions (like methane, nitrous oxide, and fluorinated greenhouse gases) (IPCC 2007a).
35 Therefore, the environmental impacts of different design techniques should be carefully
36 considered. At the same time, a general formulation is needed to evaluate the environmental
37 impact considering the entire service-life of engineering systems. Different processes in each
38 stage of the life cycle cause GHG emissions, from the manufacturing and transport of materials
39 to the construction of the system, and from the usage stage to the demolition stage. Even
40 though the greater contribution of emissions is typically due to the use stage, GHG emissions
41 due to the materials and energy production, in addition to the construction stage, play a
42 significant role in the emission reduction (Ochsendorf 2012; Yeo and Potra 2015). Some recent
43 studies focused on the assessment and the optimization of engineering systems (especially for
44 buildings) by minimizing their GHG emissions by performing a Life Cycle Assessment (LCA)
45 in terms of their environmental impact. In this sense, optimizations strategies have been also
46 proposed (Norman et al. 2006; Kumar and Gardoni 2013). The purpose of these studies is to
47 promote the reduction of GHG focusing on emissions at each stage (Seo and Hwang 2001) or
48 identify the phases causing the most significant emissions and the most affecting materials
49 (Welsh-Huggins and Liel 2017; Fay et al. 2000).

50 However, aspects of sustainability and hazard resistance are often considered separately
51 (Gardoni 2019). Therefore there is a need to base the modern designs of the engineering
52 systems on multidisciplinary considerations balancing sustainability and reliability (Gardoni et
53 al. 2016; Alibrandi and Mosalam 2017; Gardoni 2017; Murphy et al. 2018). In addition,

54 existing sustainability studies typically do not consider the deterioration of engineering systems
55 over time, which might lead to an increase of GHG emissions in the usage stage and well as
56 new emissions due to needed repairs.

57 Only a few studies have proposed improvements to LCA by considering stages associated
58 with hazard damage and deterioration in the life cycle of the engineering systems (e.g., Padgett
59 and Tapia 2013; Hossain and Gencturk 2014; Welsh-Huggins and Liel 2017) and provided
60 attempts to integrated sustainability and resilience (e.g., Menna et al. 2013; Feese et al. 2015;
61 Alirezai et al. 2016; Belleri and Marini 2016; Calvi et al. 2016; Dong and Frangopol 2016;
62 Padgett and Li 2016; Wei et al. 2016; Chhabra et al. 2017; Simonen et al. 2018; Liel and
63 Welsh-Huggins 2019; Yang and Frangopol 2019; Faber et al. 2020; Giresini et al. 2020).
64 However, there is still a need for a general stochastic formulation to assess the environmental
65 impact of the life-cycle of engineering systems.

66 This paper defines a general formulation to predict the environmental impact over time of
67 an engineering system. The assessment is performed in terms of embodied energy, energy
68 consumption, and carbon footprint during the service life of the system (which could be longer
69 than the lifespan of the system if the system needs to be rebuilt). The formulation considers the
70 possible repairs or reconstruction needed due to gradual and/or shock deterioration.

71 As an example, the proposed formulation is illustrated by computing the environmental
72 impact of a four-story building in Los Angeles, California. The formulation is useful for
73 selecting the most environmentally friendly design and construction activity to reduce
74 greenhouse gas emissions, considering the deterioration and hazard resistance of the
75 engineering system.

76 The next section reviews previous studies and methods used to develop the proposed
77 formulation. The third section presents the proposed Stochastic Life-Cycle Assessment. In the
78 fourth section, the paper analyzes the example building.

79 **2 Literature review**

80 **2.1 Life-Cycle Assessment**

81 The U.S. Environmental Protection Agency defines the well-known Life-Cycle Assessment
82 (LCA) as a “cradle-to-grave” approach (EPA 2008). LCA is the evaluation of environmental
83 emissions during a product’s entire lifespan. It analyzes the inflow of material and energy
84 processes and outflow of environmental impacts during the entire lifespan (Welsh-Huggins and
85 Liel 2017). LCA quantifies the consumption of materials, energy, and emissions into the
86 environment and evaluates their impacts and possible solutions to reduce it. There are four
87 steps in LCA (EPA 2008): 1) Goal and scope definition; 2) Life-Cycle Inventory (LCI) or
88 inventory analysis, which includes data collection and calculations to quantify the material and
89 energy transactions of the system; 3) Life-Cycle Impact Assessment (LCIA), which includes
90 evaluation of potential environmental impacts based on the LCI; 4) Interpretation of results,
91 which is the evaluation of the results of the inventory analysis and the impact assessment.

92 Recent studies include the optimization of systems by minimizing GHG emissions by
93 performing an LCA in terms of environmental impact (e.g., Seo and Hwang 2001; Park et al.
94 2013; Biswas 2014; Luo et al. 2016). The main goal is to find measures to reduce GHG
95 focusing on 1) construction materials for infrastructure, 2) building operations, and 3) material
96 transportation.

97 **2.2 Life-Cycle Energy Analysis**

98 The Life-Cycle Energy Analysis (LCEA) allows the consideration of all the energy inputs to a
99 system in its life-cycle (Ramesh et al. 2010). The analysis typically includes the following
100 phases: manufacturing, usage, and demolition. Each stage of the life-cycle of an engineering
101 system is associated with energy use. Considering the life cycle of the building allows making
102 long-term considerations. Life-cycle energy use can be distinguished as embodied energy,
103 operating energy, and demolition energy (Ramesh et al. 2010; Fay et al. 2000).

104 *Embodied energy*, as well known, includes the acquisition of the sources of energy for all
105 construction stages, including the renovation of the system. It consists of two parts: initial (i.e.
106 the energy incurred for the first construction of the system) and recurring embodied energy (i.e.
107 that for some regular maintenance and replacements).

108 *Operating energy* is the part required to ensure prescribed comfort features. For a building,
109 it is the energy for HVAC (heating, ventilation, and air conditioning), domestic hot water,
110 lighting, and running appliances.

111 *Demolition energy* takes into account the demolition stage of the construction including the
112 transportation of the waste to landfill sites and/or recycling plants (Ramesh et al. 2010).

113 Many studies show the differences between the embodied energies of different systems and
114 evaluate strategies for optimizing the energy requirements (Cole and Kernan 1996; Fay et al.
115 2000). Several studies reported that operating energy has the biggest contribution (80-90%) in
116 the life-cycle energy use of buildings followed by embodied energy (10-20%), whereas
117 demolition energy has a very small share (Cole and Kernan 1996; Ramesh et al. 2010). Even
118 though the greater contribution is due to the operating energy, embodied energy plays a
119 relevant role in emission reduction (Gardoni et al. 2003). In particular, embodied energy
120 assessment allows the evaluation of the performance in terms of energy efficiency, which
121 combines both the costs and the environmental impacts. Embodied and operating energy can be
122 reduced by acting on construction techniques or the choice of materials. As an example,
123 adopting materials that require less energy during manufacturing or locally available or re-
124 using materials and components. However, materials used for the reduction of the embodied
125 energy might have a greater impact on the use phase (Ramesh et al. 2010). Similarly, optimal
126 thermal insulation can reduce operating energy over time but it may have a greater contribution
127 to the embodied energy. The environmental impacts of different life-cycle stages are strongly
128 interdependent, as one stage may affect one or more of the others. The goals should be to reach

129 a balance between Embodied and Operating energy (Fay et al. 2000). Such balance can be
130 reached by looking at the entire building's life.

131 LCEA can be useful when considering strategies to reduce primary energy use and
132 emissions control. Primary energy includes all-natural sources (all energy products not
133 transformed), it is the energy needed to produce the energy used by the consumer. The energy
134 consumed by the customers is called end-use energy or delivered energy (Fay et al. 2000);
135 Ramesh et al. 2010). For assessing the environmental impact, considering primary energy is
136 recommended. However, energy is only a part of the environmental impact assessment and
137 Life-Cycle Assessment of buildings is needed for a more complete environmental impact
138 analysis.

139 **2.3 Life-Cycle Analysis of engineering systems**

140 Engineering systems are subject to gradual and shock deteriorations (Chanter and Swallow
141 2007; Choe et al. 2008; Kumar et al. 2009; Kumar and Gardoni 2012; Kumar and Gardoni
142 2013; Kumar and Gardoni 2014a; Kumar and Gardoni 2014b). Jia et al. (2017) proposed a
143 stochastic formulation, called Stochastic Life-Cycle Analysis (SLCA), which integrates the
144 models of the state-dependent deterioration (Jia and Gardoni 2018a,b) and the state-dependent
145 recovery and resilience analysis (Sharma et al. 2017). SLCA defines the life-cycle performance
146 of a deteriorating system in terms of an indicator or performance measure $Q(t)$, as a function of
147 time t (e.g., the reliability or functionality of the system).

148 Deterioration models (e.g., Jia and Gardoni 2018a,b) predict the system state as a function
149 of the values of the state variables $\mathbf{X}(t)$ that typically change with time due to multiple
150 deterioration events. The deterioration of the system is affected by external conditions/variables
151 at time t , defined as $\mathbf{Z}(t) = [\mathbf{E}(t), \mathbf{S}(t)]$, where $\mathbf{E}(t)$ denotes environmental
152 conditions/variables and $\mathbf{S}(t)$ indicates shocks/hazards intensity measures. The vector $\mathbf{X}(t) =$
153 $[X_1(t) \cdots X_j(t) \cdots X_{n_x}(t)]^T$ signifies the state of variables of the system at time t . The set of

154 basic variables included in this vector are material properties, member dimensions, imposed
 155 boundary conditions, etc. (Gardoni et al. 2002, 2003). The initial state variables at time $t = 0$ is
 156 $\mathbf{X}_0 = \mathbf{X}(t = 0)$. The system state changes from \mathbf{X}_0 to $\mathbf{X}(t)$ at time t due to multiple
 157 deterioration processes.

158 Sharma et al. (2017) proposed a stochastic recovery model for deteriorated (or damaged)
 159 systems. The recovery model is based on modeling the time of the recovery steps and of the
 160 occurrence time of possible disrupting shocks (e.g., seismic loads). A work plan can be
 161 developed for every recovery process, containing all the required recovery activities. By the
 162 combination of deterioration (Jia and Gardoni 2018a,b) and recovery models (Sharma et al.
 163 2017), we can compute $\mathbf{X}(t)$ at every time t during the service life of a system. Once the time-
 164 variant state variables $\mathbf{X}(t)$ are determined, the capacity of the system and the demand that a
 165 shock imposes on the system can be defined. Following Gardoni et al. (2002), the capacity of
 166 the system can be expressed as

$$C(t) = C[\mathbf{X}(t), \boldsymbol{\theta}_C] \quad (1)$$

167 where $C[\mathbf{X}(t), \boldsymbol{\theta}_C]$ is the predicted capacity at time t , and $\boldsymbol{\theta}_C$ is the vector of parameters of the
 168 capacity model. The demand due to a shock, with intensity measures $\mathbf{S}(t)$ can be written as (Jia
 169 et al. 2017; Sharma et al. 2017; Jia and Gardoni 2018a,b)

$$D(t) = D[\mathbf{X}(t), \mathbf{S}(t), \boldsymbol{\theta}_D] \quad (2)$$

170 where $D[\mathbf{X}(t), \mathbf{S}(t), \boldsymbol{\theta}_D]$ is the predicted demand at time t , and $\boldsymbol{\theta}_D$ is the vector of parameters
 171 of the demand model. The capacity and demand models can predict the time-variant system
 172 performances, measured in terms of $Q(t)$ expressed as

$$Q(t) = Q[C(t), D(t)] \quad (3)$$

173 For example as in Jia et al. (2017), Sharma et al. (2017), and Jia and Gardoni (2018a,b),
 174 $Q(t)$ could be the conditional failure probability, or fragility, at time t given the occurrence of

175 a shock with a given intensity measure or the reliability index of the system (Ditlevsen and
176 Madsen 1996; Gardoni 2017) given the limit-state function $g(t) = C(t) - D(t)$.

177 **2.4 Probabilistic assessment of structural damage**

178 The paper considers earthquakes as the main hazards. If an earthquake occurs, a repair might be
179 needed, which has a corresponding environmental impact. Therefore, a probabilistic assessment
180 of the possible structural damage is needed to then account for the contribution to the
181 environmental impact of the occurrence of a hazard and the post-hazard functionality.

182 Bai et al. (2009) proposed a probabilistic approach to compute the conditional probability
183 of having specified structural damage (or being in a certain damage state) for a given seismic
184 intensity. Their study proposes a damage state classification developed from the Applied
185 Technology Council (ATC-13) damage factors, the ATC-38 damage state classifications, and
186 the ATC-38 database of building damage. Considering d damage states, the special case of Bai
187 et al. (2009) considers $d = 4$. Table 1 provides the description of each damage state according
188 to Bai et al. (2009).

189 The probabilistic approach proposed by Bai et al. (2009) is based on the relationship
190 between the performance levels used to define fragility curves (i.e., each performance level P_L
191 is introduced in the definition of the limit state function that defines a fragility curve) and
192 damage states. Damage factors L_k are assigned to each damage state k to quantify the structural
193 damage as a percentage of the portion of the structure that needs to be replaced. To capture the
194 variability in L_k for a given damage state, L_k is assumed to be a random variable with a beta
195 distribution. Once the damage factor is defined, we obtain the total damage factor for a given
196 intensity measure $L_k|IM$. Figure 1 shows the relationship between fragility curves and damage
197 states. The figure shows that the damage states are bounded by the fragility curves.

198 The constitutional probability of being in each damage state $P_{k|IM}$ can be calculated as the
199 difference between the conditional probabilities of the bounding fragility curves P_{L1}, P_{L2} and

200 P_{L3} for a given IM. Following Bai et al. (2009), $L|IM$ is assumed to have a Beta distribution as
 201 L_k . Considering d damage states, the conditional mean and the variance can be calculated as

$$E[L|IM] = \mu_{L|IM} = \sum_{k=1}^d (L_k \cdot P_{k|IM}) \quad (4)$$

$$Var[L|IM] = \sigma^2_{L|IM} = \sum_{k=1}^d [(L_k - \mu_{L|IM})^2 \cdot P_{k|IM}] \quad (5)$$

202 where $\mu_{L|IM}$ is the conditional mean of the total damage factor for a given IM and $\sigma^2_{L|IM}$ is the
 203 conditional variance of the total damage factor for a given IM.

204 **3 Proposed Stochastic Life Cycle Assessment**

205 **3.1 Overall formulation**

206 We propose a general stochastic formulation for assessing the environmental performance over
 207 time of an engineering system in terms of its carbon footprint, embodied energy, and energy
 208 consumption. The proposed stochastic life-cycle assessment can consider shock and gradual
 209 deteriorations in the sustainability analysis and estimation of the GHG emissions throughout
 210 the lifespan of a system. The life cycle of engineering systems includes several stages, each
 211 stage is associated with energy consumption and GHG emissions. During its life cycle, the
 212 system is subject to various hazards and it is likely to need repair after a certain level of
 213 deterioration or damage, such repair provides a further contribution of energy. The analysis
 214 considers the following phases in a life cycle: construction, usage, demolition, and recovery,
 215 where the energy uses are the embodied energy, operating energy, demolition energy, and
 216 recovery energy.

217 Figure 2 presents the flow of the basic steps in the proposed formulation. The formulation
 218 starts with the input data that define the characteristics of the engineering system and the external
 219 conditions. As we defined in Section 2.3, the initial state variables at time $t = 0$ are expressed by
 220 the vector \mathbf{X}_0 ; this vector includes variables such as material properties, the geometry of the

221 system, and imposed boundary conditions. The vector of the external conditions/variables at time
222 $\mathbf{Z}(t) = [\mathbf{E}(t), \mathbf{S}(t)]$ is composed of the environmental conditions/variables, $\mathbf{E}(t)$ and
223 shocks/hazards intensity measures, $\mathbf{S}(t)$. In the System Modeling step, we model the deterioration
224 and recovery/repair cycles of an engineering system throughout its lifespan according to the
225 SLCA proposed by Jia et al. (2017), Jia and Gardoni (2018a,b), and Sharma et al. (2017). The
226 state variables and the capacity and demand models are used to predict the time-variant system
227 performance measures $Q(t)$. Finally, in the Environmental Impact Analysis, we compute the
228 energy use and related emissions at each stage.

229 **3.2 System modeling**

230 Engineering systems alternate phases of being in use or down and are subject to gradual and
231 shock deterioration. We model the system under the deterioration and repair/recovery cycles
232 using the Stochastic Life-Cycle Analysis proposed by Jia et al. (2017), Sharma et al. (2017),
233 and Jia and Gardoni (2018a,b). Taking into account the gradual and shock deteriorations, we
234 define the state variables of the structural system $\mathbf{X}(t)$ considering environmental
235 conditions/variables as well as shocks/hazards models. Once the state variables are defined at
236 every time t by combining the deterioration and recovery models, we can compute the capacity
237 of the system using Eq. (1) and the demand that the shock imposes on the system using Eq. (2).
238 In general, all models of capacity and demand that assume the state variables as input can be
239 adopted in the formulation. We model the state variables and calculate the corresponding
240 capacity and demand to predict the system state at each time t , measured in terms of $Q(t)$,
241 which can be, for example, a measure of functionality, reliability, or efficiency. In particular,
242 once the limit-state function, $g(t) = C(t) - D(t)$ is defined, we can calculate the time-variant
243 probability of failure of the system, given the occurrence of a shock with a given intensity
244 measure. At each time t , we can obtain a set of fragility curves by considering different
245 performance levels (or capacities) that define different limit states.

246 For shock deterioration, we model the occurrence times and intensities of possible shocks
 247 that can affect $Q(t)$. Specifically, considering earthquakes as shocks, we model the random
 248 occurrence of shocks as a Poisson process, which in general could be either homogeneous (i.e.,
 249 with constant occurrence rate) or non-homogeneous (i.e., with time-varying occurrence rate).
 250 More generally, any random process that can give the probability of occurrence of an event can
 251 be adopted. Given the occurrence of a shock, the intensity of the shock can be modeled using
 252 commonly used distributions for the specific intensity measure, which are typically site-
 253 dependent. Any earthquake intensity measure can be adopted in the proposed formulation. The
 254 earthquake intensity is here assumed to follow the distribution calculated using the second-
 255 order logarithmic formulation for hazard curves according to Kumar and Gardoni (2013)

$$\ln\{P[S > s]\} = a_1 + a_2 \left[\ln\left(\frac{s}{S_{min}}\right) \right]^2 \quad s \geq S_{min} \quad (6)$$

256 where $\ln(\cdot)$ is the natural logarithm, and $a_1 < 0$, $a_2 < 0$ and S_{min} are regional constants that
 257 depend on the interpolated site-dependent data. The expression in Eq. (6) is a concave parabola
 258 with the vertex at $(\ln(S_{min}), a_1)$. We use only the part of parabola where $s \geq S_{min}$ (i.e., the
 259 right portion) in which the hazard curve is monotonically decreasing. Using Eq. (6), we obtain
 260 the hazard curve with the annual rate of exceedance, that, considering a short time span (like
 261 one year) can be confused with the annual probability of exceedance. Then, considering the
 262 time span of the building we obtain the distribution used in our simulation.

263 **3.3 Environmental impact analysis**

264 Carbon Footprint, in terms of “carbon dioxide equivalent” or CO₂e, is a common metric to
 265 account for the global warming impact (GWP) of the different GHG. CO₂e quantifies, as well
 266 known, the equivalent amount of CO₂ emitted from the different GHGs and is commonly
 267 measured in tonnes of carbon dioxide equivalents (tCO₂e).

268 3.3.1 Computation of emissions

269 Once the scope is defined and the impact categories which need to be evaluated are decided
270 upon, we can compute the total amount of the CO₂e emissions at each stage, as functions of
271 time and the system state. This formulation is different from existing ones because CO₂e
272 emissions depend on time. The total amount of the CO₂e emissions (tCO₂e) when the
273 engineering system is in use is

$$CO_{2,USE}([t_0, t_f]) = \sum_{i=0}^{n-1} \Delta CO_{2,USE}(t_i, t_{i+1}) \quad (7)$$

274 where t_0 is the initial time of the usage stage, t_f is the final time of the usage stage that
275 corresponds to the lifespan of the building, n is the index of the last time step, and

$$\Delta CO_{2,USE}(t_i, t_{i+1}) = [OE(t_{i+1} - t_i) + EE_d(Q([t_i, t_{i+1}]))] \cdot CF \quad (8)$$

276 in which OE is the operating energy (kWh), EE_d (kWh) is the recurring embodied energy
277 computed in the time interval Δt needed when the engineering system is in use for maintenance
278 and replacements (this term depends primarily on gradual deterioration), and CF ($\frac{tCO_2e}{kWh}$) is the
279 conversion factor which corresponds to the equivalency factors. Since EE_d is unknown, it is
280 treated as a random variable in the proposed formulation.

281 The Operating energy evaluated in the interval Δt is expressed as

$$OE(t) = \int_{t_i}^{t_{i+1}} E_p(t) dt \quad (9)$$

282 where $E_p = \frac{E_c}{\eta}$, E_c is the energy consumption (a result of the energy simulation), and η is the
283 efficiency of the power plant producing the energy. With η we take into account the percentage
284 of power loss of the power plant. We have to consider the loss to compute the emissions in
285 terms of primary energy. The two quantities E_c and E_p refer to energy consumption (end-use
286 energy or delivered energy) and energy production, respectively. For assessing the
287 environmental impact, it is recommended to consider primary energy.

288 CO₂e emissions in the recovery stage are functions of the conditional probabilities of being
 289 in each damage state. The expected amount of CO₂e emissions (tCO₂e) in the recovery stage
 290 $CO_{2,R}$ at any time t , when a hazard occurs, can be expressed as

$$CO_{2,RE} = E[CO_{2,RE}|(IM, \mathbf{X}(t))] \quad (10)$$

291 where $E[CO_{2,RE}|IM]$ is the expected value of CO₂e for a given intensity measure.

292 Construction and recovery phases are evaluated in the same way (i.e., the recovery phase
 293 from the state of complete damage is the same as the construction phase plus a demolition
 294 phase). Therefore, the total amount of CO₂e emissions (tCO₂e) in the construction stage
 295 ($CO_{2,CS}$) can also be expressed by Eq. (10).

296 The total amount of CO₂e emissions (tCO₂e) in the Demolition Stage ($CO_{2,DS}$) is

$$CO_{2,DS} = \sum_{i=1}^m q_i \cdot I_i \quad (11)$$

297 where m is the number of materials/processes, q_i (kg) is the quantity of engineering system
 298 material/process; I_i (kgCO₂e) is the impact indicator of the material/process per unit. All the
 299 terms of this equation needed to calculate the CO₂e emissions are included in the Demolition
 300 Energy. After finding the different amount of CO₂e emissions during the whole life cycle of the
 301 engineering system, we can obtain the total carbon footprint during the life-cycle of the
 302 engineering system $CO_{2,life-cycle}$ by adding all of the contributions calculated from Eqs. (7),
 303 (10) and (11)

$$CO_{2,life-cycle} = CO_{2,CS} + CO_{2,USE} + CO_{2,DS} + CO_{2,RE} \quad (12)$$

304 3.3.2 Life cycle inventory and energy simulation

305 After determining the materials and energy processes needed for the system, we convert them
 306 into their related emissions. Conversion factors provide information on the amount of

307 pollutants discharged into the atmosphere by a process, fuel, equipment, or specific source. In
308 this analysis, the factor is expressed in tons of CO₂e per tons of material/process/energy.

309 We make an estimation of the list of activities for each stage of the system. To conduct the
310 inventory analysis in this formulation, we use the Work Breakdown Structure (WBS) to make a
311 list of all the activities. WBS is a decomposition of the project into various hierarchies
312 depending on different levels of detail (California Energy Commissions 2016). Task definition
313 can be determined through the WBS and the activity list. The activity list presents the tasks
314 required for a project. Each activity is a single work task that takes some amount of time and
315 has an identifiable start and finish times. Figure 3 illustrates the first level of a simplified WBS,
316 the entire level consists of mobilization and site preparation, substructure, superstructure, and
317 finishing. Within these levels, there may be further divisions and, finally, a list of activities is
318 obtained.

319 WBS is the starting point to estimate GHG emission and energy consumption during the
320 stage of the supply of construction materials, as well as the construction, recovery, and
321 demolition stages. Once the activity list is defined, we determine the amount and sources of
322 different construction materials, equipment (work hours and energy consumption), and
323 transport of materials. Then, we can determine the energy use and CO₂e emission required for
324 each task. Once we know the quantities of materials for each activity, we can estimate the
325 emissions from production and transport.

326 The structural damage is evaluated as a percentage of structural portion replacement of the
327 system for each damage state and related emissions. The CO₂e emissions associated with each
328 damage k , $CO_{2,k}$, are determined to estimate the total amount of emissions during the recovery
329 stage $CO_{2,RE}(t)$, where $CO_{2,RE}(t)$ is calculated based on the data of the Life-Cycle Inventory.
330 The term $CO_{2,k}$ for each damage state can be expressed with the following expression:

$$CO_{2,k} = \sum_{i=1}^m q_i \cdot I_i + E_C \cdot CF, \quad (13)$$

331 where m is the number of materials/energy/processes, q_i (ton) is the quantity of engineering
 332 system material/process, I_i ($\frac{tCO_2e}{ton}$) is the impact indicator of material/process per unit, E_C
 333 (kWh) is the energy for the construction stage; CF ($\frac{tCO_2e}{kWh}$) is the conversion factor. All the
 334 terms are included in the definition of Embodied energy. The impact indicator I_i provides
 335 information on the global warming impact of each component. It includes emissions due to the
 336 production and manufacture of materials. The embodied energy and CO_2 emissions are defined
 337 deterministically; however, the input variables in Eq. (13) could be modeled as random
 338 variables. The energy in the usage stage is given by software like Energy-Plus, VisualDOE, e-
 339 Quest, DesignBuilder, Ecotect, or by available data (Ramesh et al. 2010). The Operating energy
 340 of a building is given by electricity and fuels for heating, sanitary water, lighting, and
 341 appliances.

342 3.3.3 Modeling of the Damage State

343 After modeling the system, as mentioned in Section 3.2, the system state at any time t is
 344 represented in terms of the performance indicator $Q(t)$, which changes with time due to
 345 gradual and shock deteriorations. A recovery operation is needed when the indicator
 346 deteriorates beyond a specified (or desired) limit (Jia et al. 2017; Sharma et al. 2017; Jia and
 347 Gardoni 2018a,b). We describe the damage of the system at a given time t using the Probability
 348 Mass Function (PMF) of $Q(t)$, which gives the probability that $Q(t)$ takes a specific value
 349 within a set of possible values. We define different recovery strategies corresponding to each
 350 damage state. We also account for the emissions from the energy and materials consumed by
 351 deterioration and consequent recovery.

352 For the k^{th} damage state, we define the mean value of the emissions and a corresponding
 353 confidence interval. We then calculate the probabilistic GHG emissions corresponding to each

354 recovery strategy. We assume a Beta distribution for the emissions associated with each
355 damage state and we draw samples from these distributions, $CO_{2,k}$. The expected value of the
356 emissions, $CO_{2,RE}$, for a given intensity measure at time t , considering the k^{th} damage state, to
357 account the emissions for the recovery stage $CO_{2,RE}$ is expressed as

$$E[CO_{2,RE}|IM] = \sum_{k=1}^s CO_{2,k} \cdot P_{k|IM}, \quad (14)$$

358 where $E[CO_{2,RE}|IM]$ is the expected value of CO₂e for a given intensity measure at time t , s is
359 the number of damage states, $CO_{2,k}$ is the emissions associated with each damage state. Eq.
360 (14) is needed for computing the amount of CO₂e emission in the recovery stage.

361 **4 Example**

362 The proposed formulation is illustrated in this section considering a hypothetical four-story
363 office building made in reinforced concrete and located in Los Angeles, California. We apply
364 the general formulation described earlier to calculate the total carbon footprint during the life
365 cycle of the building. This example includes the energy use of construction, usage, demolition,
366 and recovery. Accordingly, the energy uses for each phase are embodied energy, operating
367 energy, demolition energy, and recovery energy.

368 **4.1 Input data and building modeling**

369 The space frame has a floor area of 120 by 180 ft with six frame lines resisting lateral loads in
370 each direction, located in seismic design Category D. So the total amount of slabs for the four
371 floors is 86,400 ft² (which means 8,026 m²). In this example, we compare the environmental
372 impact of the life-cycle of a seismically designed (with the current design standards, Haselton
373 et al. 2008) building and a non-seismically designed building, consisting of only a vertical load
374 resisting system (Williams et al. 2009).

375 The service life of the buildings is assumed to be 100 years. We assume to only consider
376 shock deterioration due to earthquakes. Because of this, the operating energy is constant for

377 each Δt when the building is in use, and we do not consider recurring embodied energy. Also,
378 we assume that the system state goes back to the undamaged state at the end of the recovery
379 activities and that the building is repaired quickly with respect to the frequency of earthquakes
380 (i.e., each repair is short with respect to the time between consecutive earthquakes.)

381 The fragility curves are on three different performance levels: P_{L1}, P_{L2}, P_{L3} (Hazus
382 Technical Manual 2019). Table 2 reports the parameters mean, μ and standard deviation, σ for
383 each performance level of the fragility curves used for the two different seismic design levels.

384 The occurrence of earthquakes (considered only in terms of mainshocks) is given by a
385 homogeneous Poisson process, with a constant occurrence rate (Kumar and Gardoni 2012). The
386 earthquake intensity Peak Ground Acceleration (PGA) is calculated with the second-order
387 logarithmic formulation for hazard curves according to Kumar and Gardoni (2013) expressed
388 by Eq. (6). For Los Angeles, the values of a_1, a_2 and S_{min} are found to be $-1.949, -0.2688$
389 and 0.005 , respectively.

390 **4.2 Environmental impact analysis**

391 The environmental impact analysis is performed by considering first an inventory analysis and
392 then the probabilistic assessment of structural damage. The carbon footprint is expressed in
393 terms of tonnes of carbon dioxide equivalents (tCO₂e).

394 **4.2.1 Inventory Analysis**

395 Before calculating the CO₂e, we conduct the inventory analysis described in Section 3.3.2. The
396 first step is to define the Work Breakdown Structure (WBS) for the construction stage. The
397 WBS is defined using data and activities from the RSMeans database (RS Means Company
398 2008). Consequently, we can determine energy and CO₂e for each task. CO₂e emissions are
399 evaluated with the impact indicator associated with each material/process and with the
400 conversion factor for the energy contribution. The processes include the acquisition of natural
401 resources, manufacturing of materials, and fuel combustion due to the transport of materials to

402 the construction site. Distances are assumed by considering the suppliers/retailers available
403 near the construction site. The impact indicators of material/process per unit are evaluated with
404 SimaPro LCA Code (SimaPro), not reported here for the sake of brevity and available in Lanza
405 (2017).

406 Once the WBS is defined for the construction stage, we define a work breakdown structure
407 for each damage state to account for a mean value of emissions (estimating the energy use, the
408 materials required for each level of damage). We consider four different damage states:
409 insignificant, moderate, heavy, and complete. To define WBS for insignificant, moderate,
410 heavy, and complete damage state we take into account a percentage of the
411 material/process/activity of the WBS for the construction phase and following the WBS of the
412 damage considered. Figure 4 shows a schematic Work Breakdown Structure for each damage
413 state. The $CO_{2,k}$ and CO_{2e} emissions associated with the construction stage are computed using
414 Eq. (13). Table 3 shows the parameters of the beta distribution associated with each damage
415 state.

416 The building materials inventory was conducted following the design drawings. We
417 estimate the energy consumption in the usage stage by performing an energy simulation
418 following the requirements of the U.S. and the State of California (Alibrandi and Mosalam
419 2017; California Energy Commissions 2016). The usage stage includes the GHG emissions
420 associated with appliances, lighting, computing, office, air conditioning, lifts, fans, heating, etc.
421 All these aspects are taken into account by eQuest, Quick Energy Simulation Tool (Hirsch
422 2010) which provides the energy consumption E_C required for maintaining comfort conditions.

423 **4.2.2 Probabilistic assessment of structural damage**

424 The damage state is modeled by using the probabilistic assessment of structural damage
425 explained in Section 2.4. Figure 5 shows the probability mass function (PMF) of the

426 performance indicator $Q(t)$ that gives the probability of being in the four different damage
427 states (insignificant, moderate, heavy, and complete) as a function of time t .

428 The conditional probability of being in each damage state for a given IM is the difference
429 between the fragility curves in Table 2. Figure 6 shows the probabilities of each damage state
430 depending on PGA. The seismically designed building is characterized by Insignificant damage
431 for $PGA < 0.3 g$, Moderate damage for $0.3 g < PGA < 0.6 g$ and Complete Damage for
432 $PGA > 1.5 g$. For the non-seismically designed building, the Insignificant and Moderate
433 damage ends up to $PGA \sim 0.3 g$, Heavy and Complete damage for $PGA > 0.3 g$. We can
434 now compute the environmental impacts due to the needed repairs after the damage caused by
435 an earthquake. The expected value of emissions for a given IM for the recovery stage $CO_{2,RE}$
436 can be calculated using Eq. (14).

437 **4.2.3 Computation of emissions at each stage**

438 We consider the conversion factor (CF) from *The Emissions & Generation Resource Integrated*
439 *Database (eGRID)*. Since the State of California's annual CO₂ equivalent total output emission
440 rate is 555.400 lb/MWh, the same CF is considered for the two buildings.

441 The total amount of the CO_{2e} is obtained from Eq. (7) and is assumed to be the same for
442 both buildings. For the case study, the comparison is just among different seismic design levels
443 of the two buildings. Figure 7 shows the annual GHG emissions in the usage stage for the
444 seismically and the non-seismically designed building.

445 The Operating energy consumption can be calculated with Eq. (9), and it is assumed to be
446 constant in (t_i, t_{i+1}) . The time interval is taken to be one year. The energy production to
447 compute the emissions is $E_p = \frac{E_C}{\eta}$. The efficiency coefficient η takes into account the
448 percentage of power losses of the power plant, considering the characteristics of the California
449 power plants, where the percentage power losses of the power plant are calculated based on

450 EIA data by the total net generation and estimated losses, as in U.S. Energy Information
451 Administration technical documents (2012 and 2014).

452 CO_{2e} emissions in the construction ($CO_{2,CS}$), the recovery stage ($CO_{2,R}$), the Demolition
453 Stage ($CO_{2,DS}$) are computed according to Eqs. (10), (11), (13), and (14) and by inventory
454 analysis data. Eq. (12) provides the total carbon footprint during the life cycle of the building
455 $CO_{2,life-cycle}$ adding all the contributions. A Monte Carlo simulation is made to account for
456 different sources of uncertainties: fragilities curves, earthquakes occurrence, distribution of
457 PGA and $CO_{2,K}$ (43,817 simulations for the seismically designed building and 22,789
458 simulations for the non-seismically designed building).

459 **4.3 Results**

460 The main results show that 60% of the emissions during the life cycle occur during the usage
461 stage for the non-seismically designed and 80% for the seismically designed building,
462 respectively. As expected, the percentage difference by about 20% is due to the higher recovery
463 required for a non-seismically designed building. The non-seismically designed building results
464 in a greater amount of emissions due to repair than the seismically designed building.

465 The comparison for the entire life-cycle between the non-seismically and seismically
466 designed building is proposed. Figure 8 shows a cumulative plot of (a) the tCO_{2e} of seismically
467 designed building, and (b) the non-seismically designed building due to repairs, considering (as
468 an example) 30 runs of the Monte Carlo simulations. The figure also provides a cumulative plot
469 of the yearly mean value of CO_{2e} over all the Monte Carlo simulations.

470 From the expected value of CO_{2e} emissions when an earthquake occurs - for each
471 realization of the Monte Carlo simulation - we can notice that the consideration of earthquakes
472 results in a greater amount of emissions throughout the lifetime of the system compared to the
473 non-consideration of earthquakes. From Figure 8 one can infer that the consideration of the
474 earthquake for the seismically designed building implies a lower increase of the emissions than

475 the non-seismically designed building. The functional unit used is m^2year . Provided that the
476 graphs of Figure 7 (c) and (d) are almost linear, it is possible to obtain an estimation of the
477 results in terms of the functional unit given by constant values. The seismically designed
478 building shows (c) $37,9 \text{ kgCO}_2\text{e}/\text{m}^2\text{year}$ with an earthquake and $31,3 \text{ kgCO}_2\text{e}/\text{m}^2\text{year}$ without
479 an earthquake. The non-seismically designed building shows (d) $45,2 \text{ kgCO}_2\text{e}/\text{m}^2\text{year}$ with
480 earthquake and $33,9 \text{ kgCO}_2\text{e}/\text{m}^2\text{year}$ without earthquake.

481 Figure 9 shows the cumulative plot of a relevant realization of the Monte Carlo simulation
482 that represents the total GHG emissions for the seismically and non-seismically designed
483 buildings. The results from the comparison show that a safer structure may mean a smaller
484 environmental impact for long-term service life. This invalidates the misconception that safer
485 usually means less environmentally friendly, largely based on the initial impact of the
486 construction. Construction of reliable systems may incur higher environmental impact in short
487 term, compared to the environmental impact of unsafe systems, however, as the service life of
488 the system increases the overall environmental impact decreases. In the long run, constructing
489 safe may also mean constructing something more environmentally friendly. For example, if we
490 look at safer structures, it means fewer repairs over a long time for a seismic region.

491 **5 Conclusions**

492 This study proposed a general stochastic formulation for the life cycle assessment of the
493 environmental impacts of engineering systems. The proposed formulation is a useful evaluation
494 tool for making decisions on the selection of the more environmentally friendly design and
495 construction process, to reduce greenhouse gas emissions, considering the hazard resistance of
496 the engineering systems. Decisions made in structural design may have long term sustainability
497 and resilience impact during the life cycle of the system. This formulation is different from the
498 existing ones because the environmental impact is assessed as time-dependent. The proposed

499 formulation can be used for a wide variety of typical engineering system configurations and
500 uses.

501 The example shows how the formulation works, we made some simplifying assumptions
502 based on the available information. To implement the proposed procedure, missing elements
503 may be added. The example includes structural uncertainties. Future work can be developed in
504 order to take into account uncertainties in calculating the embodied energy and correspondent
505 CO₂ emissions. The results show that more than 70% of the emissions come from the usage
506 stage of the building. Efforts are needed to increase the building's energy efficiency and reduce
507 energy consumption during its use phase. The energy consumption and the carbon footprint due
508 to material manufacturing account for more than 80 % of the total construction stage.
509 Renewable energy in the usage stage, recycled materials, and innovative manufacturing
510 technology in the recovery and construction stage could help reduce emissions.

511 The consideration of hazards (earthquakes in the case study) results in a greater amount of
512 emissions throughout the lifetime of the system compared to the non-consideration of hazards.
513 Probable hazards and consequent recoveries should thus be included in sustainability analysis.
514 By comparing the seismically designed building with the non-seismically designed building,
515 the results show higher emissions at the end of the service life for the non-seismically
516 retrofitted building. The results from the comparison show that a safer structure means a
517 smaller environmental impact considering long-term service life.

518 **6 Data availability statement**

519 Some or all data, models, or codes that support the findings of this study are available from the
520 corresponding author upon reasonable request.

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Tables

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Table 1 Damage state descriptions proposed by Bai et al. (2009)

Damage state	Description
(I) Insignificant	Damage requires no more than cosmetic repair. Nonstructural repairs are necessary. For nonstructural elements, repairs could include spackling, partition cracks, picking up spilled contents, putting back fallen ceiling tiles, and righting equipment.
Moderate (M)	Repairable SD has occurred. The existing elements can be repaired essentially in place, without substantial demolition or replacement of elements. For nonstructural elements, repairs would include minor replacement of damaged partitions, ceilings, contents, and equipment or their anchorages.
Heavy (H)	While the damage is significant, the structure is still standing. SD would require major repairs, including substantial demolition or replacement of elements. For nonstructural elements, repairs would include major replacement of damaged partitions, ceilings, contents, equipment, or their anchorages.
Complete (C)	Damage is so extensive that the repair of most structural elements is not feasible. The structure is destroyed or most of the structural members have reached their ultimate capacities.

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Table 2 Parameters of the fragility curves.

Seismic design level	P_{L1}		P_{L2}		P_{L3}	
	μ	σ	μ	σ	μ	σ
Seismically designed building	0.27	0.64	0.73	0.64	1.61	0.64
Non-Seismically designed building	0.13	0.64	0.26	0.64	0.43	0.64

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678

Table 3 Parameters of the Beta distribution associated with each damage state.

Damage State (K)	Mean value (tCO ₂ e)	Standard deviation
Insignificant damage	16.71	3.4
Moderate damage	641.34	35.03
Heavy damage	1,407.23	78.69
Complete damage	3,853.92	217.68

Table 1 Damage state descriptions proposed by Bai et al. (2009) [5]

Damage state	Description
Insignificant (I)	Damage requires no more than cosmetic repair. Nonstructural repairs are necessary. For nonstructural elements, repairs could include spackling, partition cracks, picking up spilled contents, putting back fallen ceiling tiles, and righting equipment.
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Complete (C)	Damage is so extensive that the repair of most structural elements is not feasible. The structure is destroyed or most of the structural members have reached their ultimate capacities.

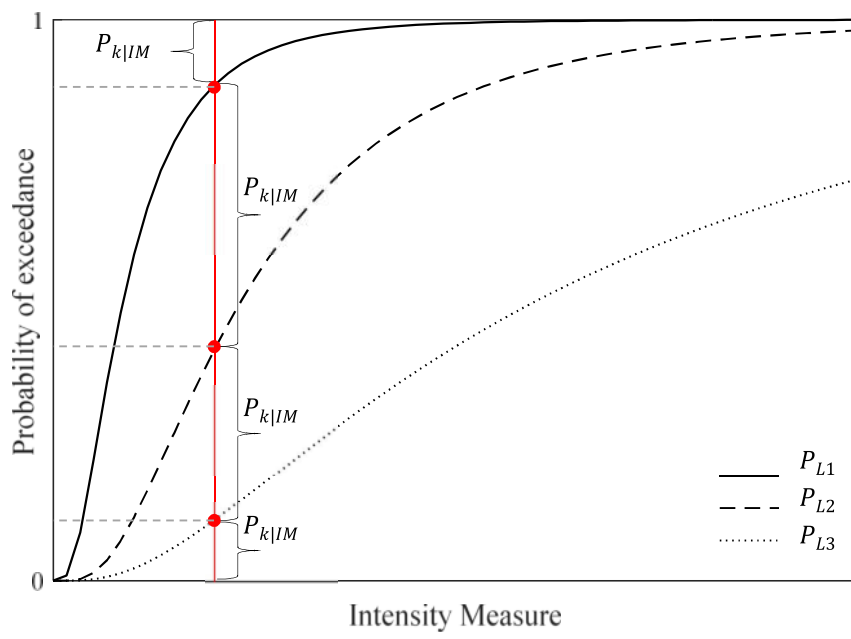
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Figure 1 Illustration of the relationship between fragility curves and damage states



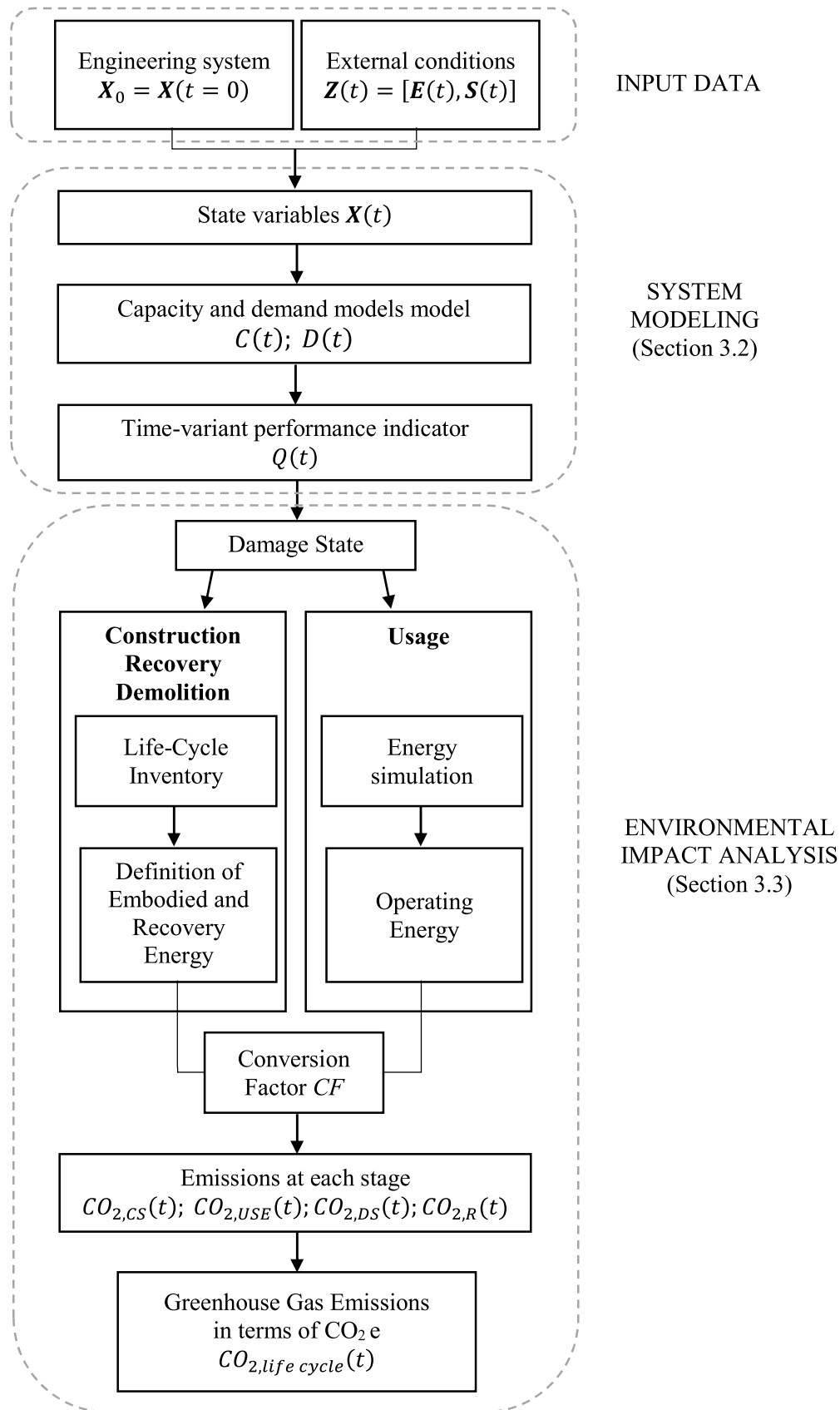


Figure 2 Flow of steps in the proposed formulation

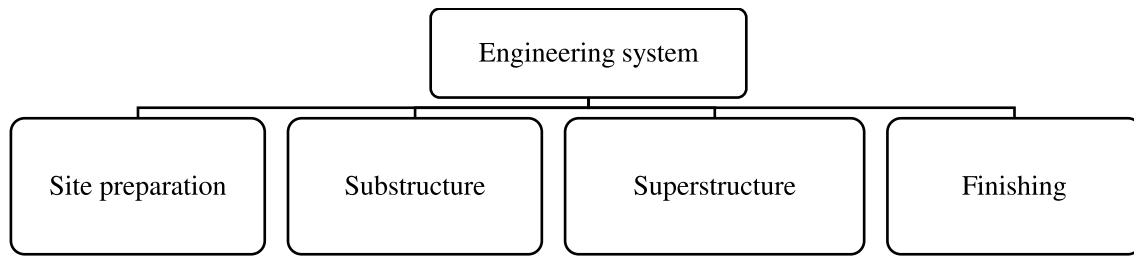


Figure 3 Illustration of a simplified WBS of a generic engineering system.

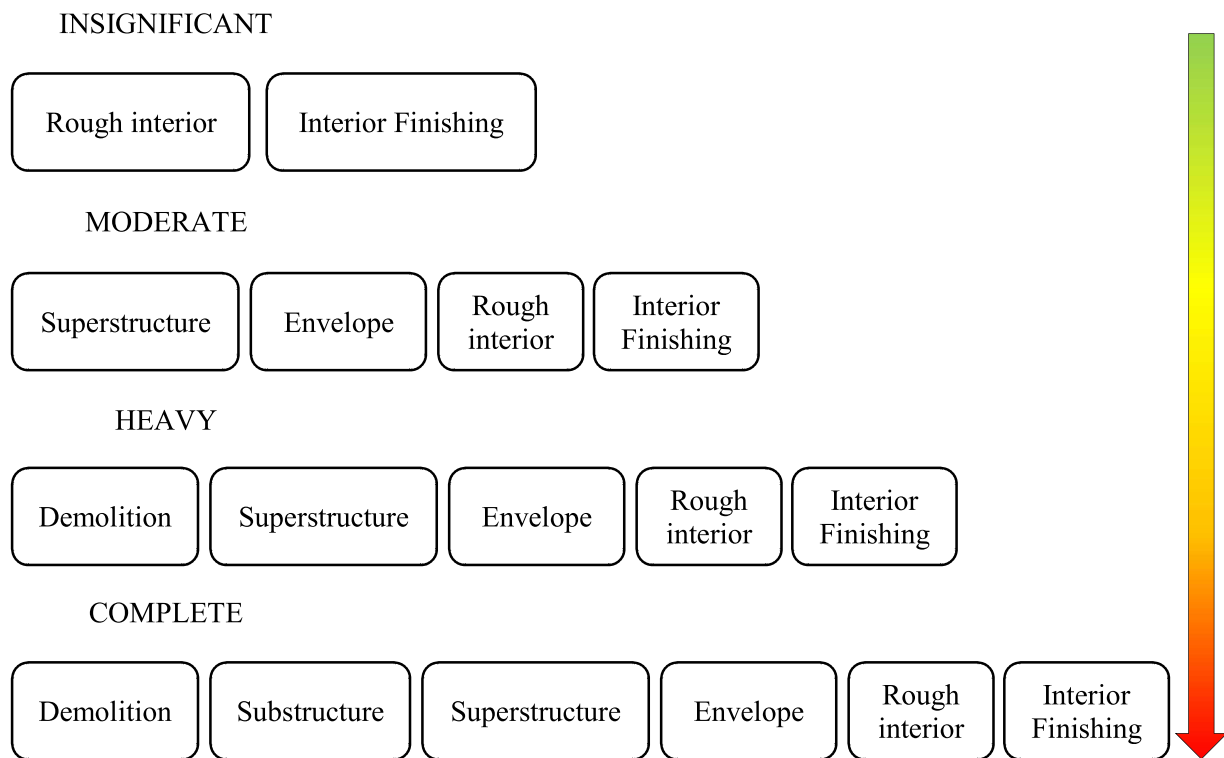


Figure 4 Schematic illustration of the Work Breakdown Structure for each damage state:

insignificant, moderate, heavy, and complete.

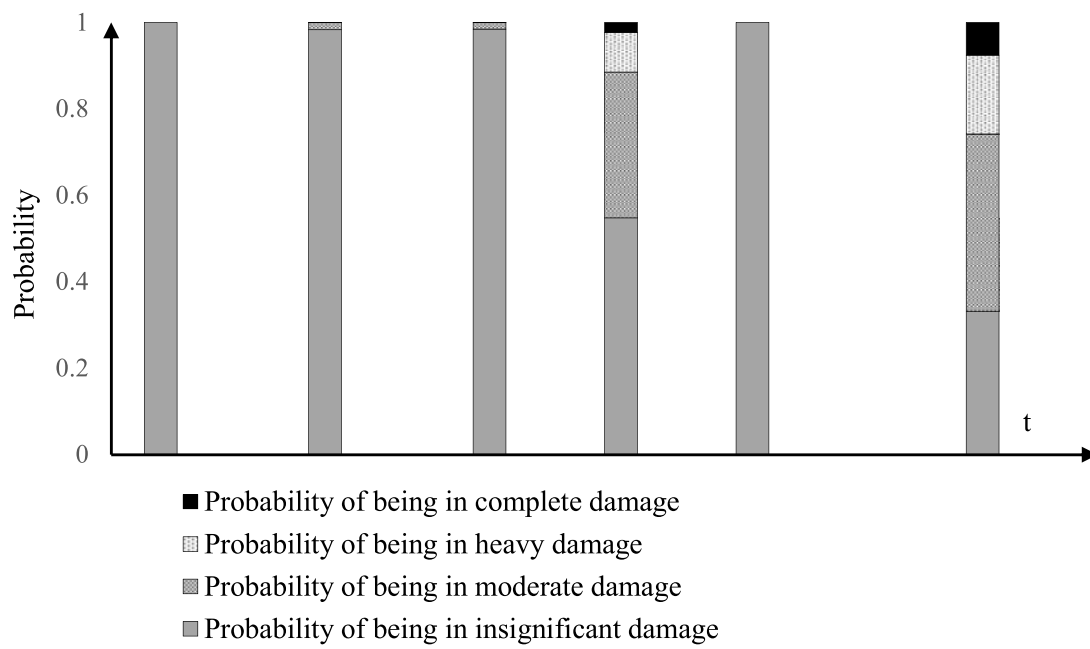


Figure 5 Probability mass function of the performance indicator $Q(t)$.

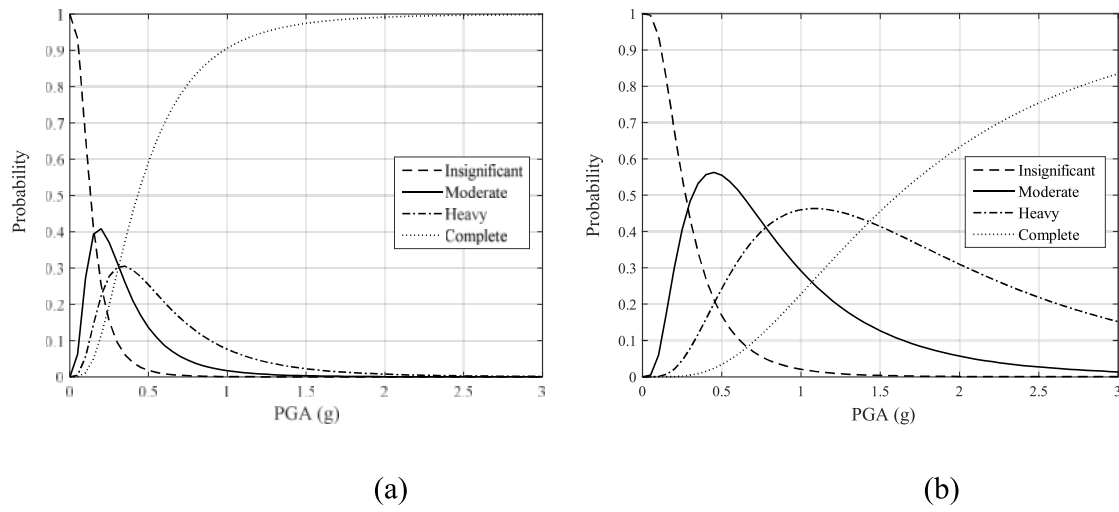


Figure 6 Probabilities of each damage state as a function of PGA for the seismically designed building (a) and the non-seismically designed building (b).

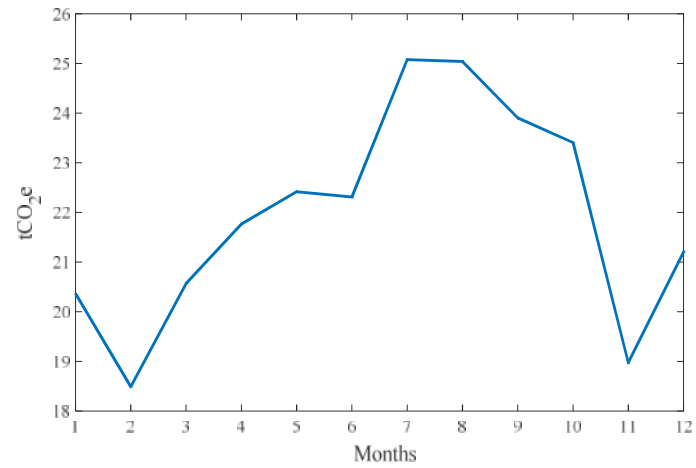


Figure 7 Annual GHG emissions in the usage stage for both seismically and non-seismically designed building.

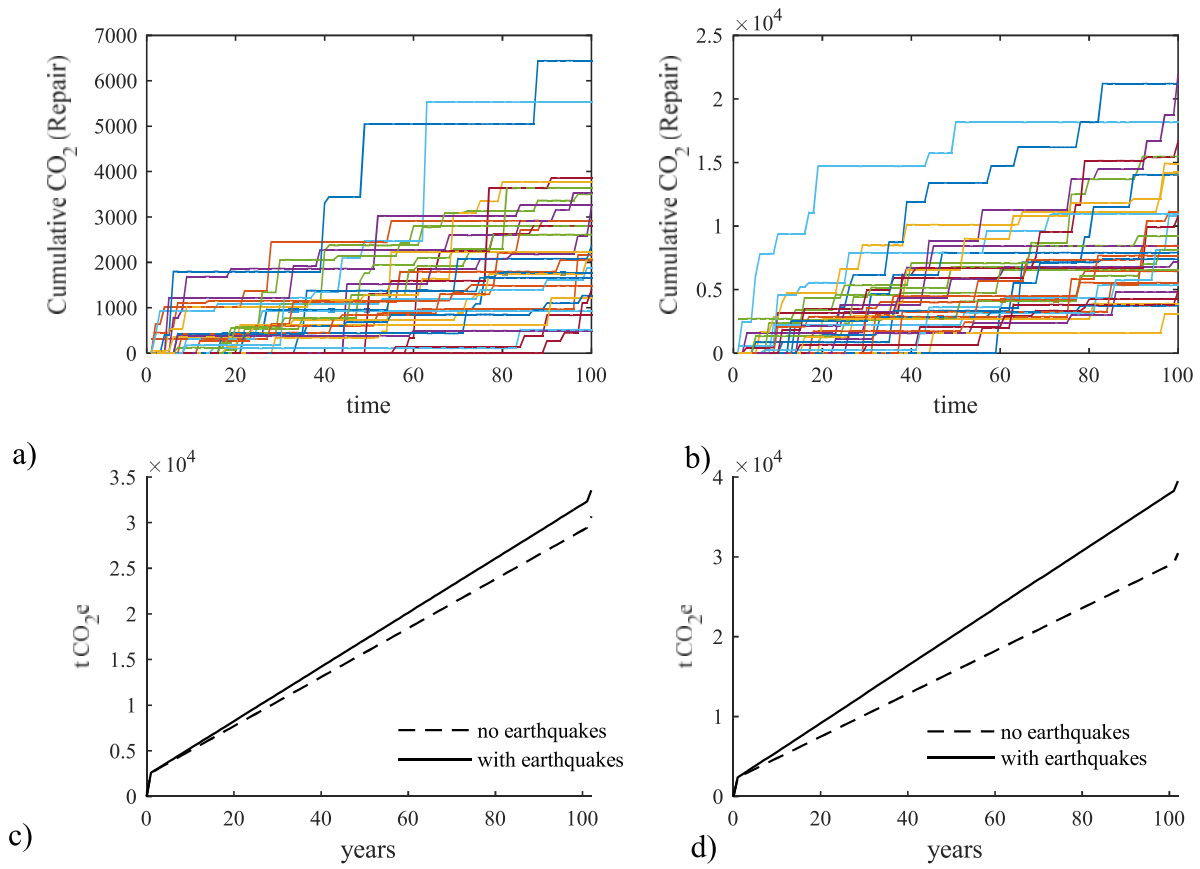


Figure 8 Sample realizations and means of the cumulative CO_{2e} emissions due to repairs for the seismically designed building (a, c) and the non-seismically designed (b, d) building.

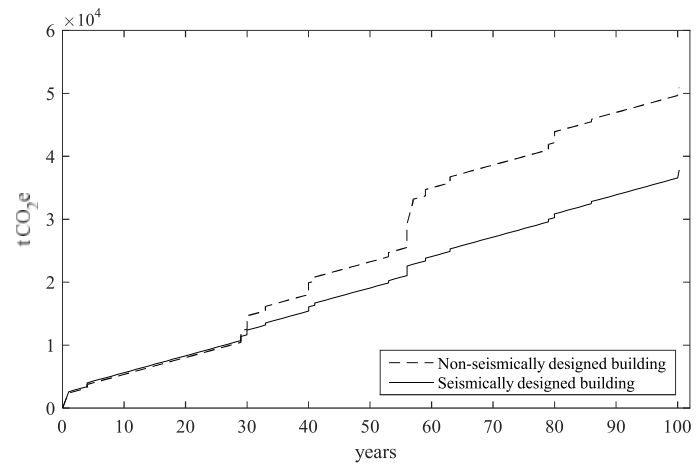


Figure 9 Total GHG emissions for non-seismically designed building and seismically design building.

LIST OF CAPTIONS

Figure 1 Illustration of the relationship between fragility curves and damage states

Figure 2 Flow of steps in the proposed formulation

Figure 3 Illustration of a simplified WBS of a generic engineering system.

Figure 4 Schematic illustration of the Work Breakdown Structure for each damage state: insignificant, moderate, heavy, and complete.

Figure 5 Probability mass function of the performance indicator $Q(t)$.

Figure 6 Probabilities of each damage state as a function of PGA for the seismically designed building (a) and the non-seismically designed building (b).

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Figure 8 Sample realizations and means of the cumulative CO_{2e} emissions due to repairs for the seismically designed building (a, c) and the non-seismically designed (b, d) building.

Figure 9 Total GHG emissions for non-seismically designed building and seismically design building.

Table 1 Damage state descriptions proposed by Bai et al. (2009) [5]

Table 2 Parameters of the fragility curves [26]

Table 3 Parameters of the Beta distribution associated with each damage state.